GENERAL NOTES

1. All work shall be coordinated with existing conditions and shall conform to the project contract documents and the New York City Building Code, latest edition.

2. Contractor shall provide temporary shoring, bracing, and sheeting, and make safe all walls and adjacent property as project conditions require. Shop drawings shall be submitted for approval for all temporary shoring and bracing.

3. Dimensions of existing construction given in drawings are based on information given in various design and construction documents provided by the Owner and limited field observations and measurements. The contractor shall verify all information pertaining to existing conditions by actual measurement and observation on site. All discrepancies between actual conditions and those shown in the contract documents shall be reported to the engineer for his evaluation before the affected construction is put in place.

FOUNDATIONS

1. Building foundations shall bear on undisturbed soil having minimum bearing capacity of 4000 psf. Adequacy of bearing stratum shall be verified in field prior to placing concrete. Where the elevation of bottom of footings, walls, or piers must be lowered to achieve proper bearing values, the affected walls or piers shall be extended as required or as indicated on the drawings or footing thicknesses may be increased as required.

2. Do not place backfill against cellar walls until they have attained their 28-day strength unless proper temporary bracing has been installed in a manner approved by the architect. Bracing shall conform to the requirements of General Note 2.

Fill

1. Refill excavated area with gravel or crushed stone. Backfill shall be placed on undisturbed soil and compacted to 95 percent of maximum density at optimum moisture content.

BRICK MASONRY

1. Where indicated, provide materials and construction which are identical to those of assemblies whose fire endurance has been determined by testing in compliance with ASTM E119 by a recognized testing and inspection organization or by another means, as acceptable to authority having jurisdiction.

2. Obtain masonry units, mortar ingredients, cementitious components, and aggregates of uniform quality, each from a single source.

3. The existing brick masonry work in place shall serve as the standard of quality for new and patching work.

4. Submit manufacturer's product data for each type of masonry unit, accessory, and other manufactured products, including certification that each type complies with specified requirements.

5. For approval of materials and match with existing materials, submit samples of face brick in each exposed color and texture to be expected in the completed work. For color selection and approval, submit cured mortar samples in plastic or metal channels. Resubmit until accepted.

6. New face brick: Type, size, texture, and color shall exactly match existing face brick. use only face brick that has been approved by the engineer. Unless otherwise specified, comply with ASTM C62, grade SW for common face brick. (3000 psi minimum compressive strength for average of 5 units, 2500 psi minimum for one unit.)

7. Mortar and grout materials:
   A. Portland cement: ASTM C150, type I, except type III may be used for cold weather construction. Use natural color or white cement as required to match existing mortar.
   B. Hydrated lime: ASTM C217, type S.
   C. Aggregates for mortar: ASTM C144, except for joints less than 1/4" use aggregate with 100% passing the #16 sieve; clean, washed and free from iron and other impurities.
   D. Water shall be clean and potable.
   E. Masonry cement: ASTM C91; a mixture of one part portland cement to one part pulverized limestone, containing an air-entraining agent to entrain not more than 15 per cent by volume; match existing mortar colors. Use only when approved, when existing mortar colors cannot be matched using portland cement mortar.

STRUCTURAL STEEL

1. All structural steel work shall conform to the following standards:
   A. AISC "Specification for the Design, Fabrication, and Erection of Structural Steel for Buildings", latest edition.
   B. American Welding Society "Code for Welding in Building Construction," (AWS D1.1) latest edition.

2. All W shapes shall conform to ASTM A572 Grade 50 or ASTM A992 with a minimum yield strength of 50 ksi unless otherwise noted.

3. All L and plates shall conform to ASTM A36 with a minimum yield strength of 36 ksi unless otherwise noted.

4. All hollow structural sections are to be cold-formed ASTM A500 Grade B with a minimum yield strength of 46 ksi.

5. Beam to girder connections shall be standard AISC framed connections unless otherwise noted.

6. Bolted connections shall use ASTM A325 bolts and are to be slip-critical unless otherwise noted.

7. Welded connections shall be performed by certified welders licensed by the Commissioner of Buildings. Welding electrodes shall be class E70XX. Use low-hydrogen electrodes for A572, Grade 50 steel.

8. All beams shall be fabricated with natural camber up.

9. Shop and erection drawings shall be submitted to the structural engineer for review and approval. No fabrication of steel shall commence without approved shop drawings.

10. Oversize or slotted holes shall not be used for any connections unless specifically indicated on the drawings or approved in writing by the engineer.

11. Cuts, holes, coping, etc. required for work of other trades shall be shown on the shop drawings and made in the shop. Cuts or burning of holes in structural steel members in the field will not be permitted.

STEEL DECK

1. All metal deck work shall conform to the AISI "Specification for the Design of Cold-Formed Steel Structural Members," latest edition.

2. All metal deck units and accessory items shall be formed from steel sheets conforming to ASTM A653, with a minimum yield strength of 33,000 psi. Before forming, the steel sheet shall receive a protective metal coating of zinc conforming to ASTM A653, Grade 60.

3. Except as otherwise noted, edge laps shall be connected with 3/4"Ø fusion welds at a spacing sufficient to provide sufficient diaphragm strength to maintain building alignment and to sustain local construction loads without distortion or separation, or equivalent steel fasteners. Maximum spacing shall be 3'-0" on center.

4. Except as otherwise noted, deck shall be attached to structural steel by 3/4"Ø fusion welds or equivalent steel fasteners, at 12" on center at end and interior supports perpendicular to the deck span and at edge and interior supports parallel to the deck span.

5. All unframed openings in deck shall be reinforced as follows:
   A. Holes less than 8" maximum width: 18 gauge flat sheet extending 8" minimum beyond hole in all directions.
   B. Holes 8" to 13" in width: 16 gauge flat sheet extending 8" minimum beyond hole in all directions.
   C. Holes larger than 13" in width shall be framed.
   D. All reinforcing shall be welded to the top side of deck. Reinforcing plates shall extend at least 3" beyond next full metal deck rib.

6. All metal deck shall be shored until concrete has achieved 50% of its design strength.

FRAMING LUMBER

1. All framing lumber shall conform to the following governing standards:
   A. American Institute of Timber Construction, "Timber Construction
   B. National Forest Products Association, "National Design Specification for Wood Construction," latest edition.

2. Framing lumber shall be of the following minimum grade and species for the specified use. All lumber shall be grade-stamped by a recognized grading agency and shall be surface dry.

   Manufactured Wood Products
   Laminated-veneer-lumber beams: Trus-Joist "Microllam" (Fb=2750 psi) or approved equivalent.

3. Provide cross-bridging or blocking at maximum 8'-0" o.c. for all joists. No joists shall be cut or notched without approval.

4. Blocking for floors or roofs framed with engineered wood products shall be timberstrand or equivalent framing member.

TIMBER CONNECTORS

1. Joist headers, cross-bridging, and connectors for wood construction shall be galvanized steel manufactured by United Steel Products, Simpson, or approved equivalent. Special nails supplied by manufacturer shall be used for required nailing.

2. Where joists are flush-framed to headers, use approved joist hangers or bridle irons.

PLYWOOD

1. Plywood sheathing shall be APA grade stamped for the specified span, shall be made with exterior glue, and shall be of the following thickness:
   Roof: 5/8
   Floors: 3/4"
   Walls: 1/2"

2. Index stamp shall be visible on all sheets.

3. All plywood shall be glue-nailed to floor joists an elastomeric construction adhesive that conforms to APA specification AFG-01 or ASTM D3498 (B.F. Goodrich PL400 or approved equal).

4. Use plyclips or other edge support as required for plywood sheathing.

5. Leave 1/16" space at all plywood panel end joints and 1/8" space at all panel edge joints.

6. Floor sheathing shall be installed continuous over two or more spans with the long dimension across supports.

SPECIAL INSPECTION

1. Special inspections required by the New York City Building Code shall be performed by a testing agency provided by the contractor and approved by the owner for the following items:

   A. Structural Steel - Welding;
   B. Structural Steel - Erection & Bolting;
   C. Wood;
   D. Structural Safety - Structural Stability;
   E. Fire Resistive Materials;
   F. Concrete Test Cylinders;
   G. Concrete Design Mix;
   H. Excavation, Sheeting, Shoring, and Bracing

2. Progress inspections required by the New York City Building Code shall be performed by a testing agency provided by the contractor and approved by the owner for the following items:

   A. Preliminary;
   B. Final.

3. The testing agency for controlled inspection shall file all appropriate forms with the Department of Buildings.

CONCRETE

1. All concrete work shall conform to the ACI "Building Code Requirements for Reinforced Concrete" (ACI 318), latest edition.

2. All concrete, unless otherwise noted, shall be normal-weight concrete with a minimum ultimate compressive strength of 4000 psi at 28 days.

3. Reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60. Reinforcing steel shall be detailed according to the ACI "Manual of Standard Practice" (ACI 315), latest edition.

4. Welded wire fabric (WWF) shall conform to ASTM A185, with a minimum ultimate tensile strength of 70,000 psi.

5. Provide minimum temperature reinforcing, as required by ACI 318, in all slabs and walls where reinforcement is not indicated on drawings.

6. Coordinate locations and dimensions of all openings inserts, weld plates, pipe sleeves, curbs, and other items to be embedded in concrete with architectural and mechanical drawings. Minimum concrete between sleeves shall be 6".

7. Contractor shall submit concrete design mixes to structural engineer for review and approval.

8. All concrete shall be placed monolithically unless otherwise noted.

9. If reinforcement bars are required to be spliced, the splice length shall comply with applicable sections of the ACI 318-08 code for tension splices. The location and length of the splices shall be shown on shop drawings and submitted to the structural engineer for review.

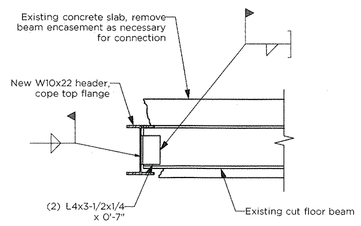
10. All grout shall be non-shrink with a minimum compressive strength of 5000 psi.

11. Provide clearance from face of concrete to reinforcement as follows:
    Slabs: 3/4"
    Concrete cast against earth: 3"
    Footings: 3"
    Beams: 1-1/2"

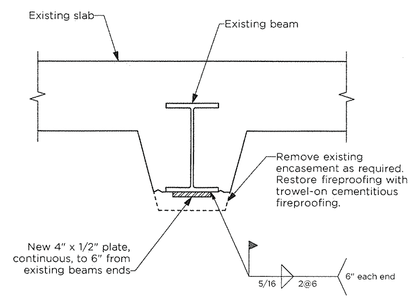
LOAD SCHEDULE

	FLOOR LEVEL*				
MATERIAL	1st Floor	2nd (Existing)	2nd (New)	Roof	Roof
FLOOR / ROOF FRAMING	6 PSF	10 PSF	6 PSF	10 PSF	5 PSF
CEILING AND HUNG	10 PSF	10 PSF	10 PSF	10 PSF	5 PSF
ROOFING	-	-	-	10 PSF	5 PSF
CONCRETE SLAB	100 PSF	-	75 PSF	-	-
FLOORING & FINISHES	10 PSF	10 PSF	10 PSF	-	-
DEAD LOAD	126 PSF	30 PSF	101 PSF	30 PSF	15 PSF
LIVE LOAD	100 PSF	50 PSF	50 PSF	30 PSF	30 PSF
TOTAL LOAD	226 PSF	80 PSF	151 PSF	60 PSF	45 PSF

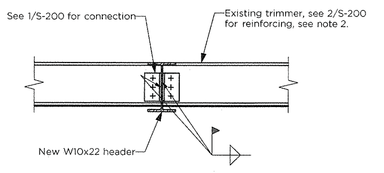
* Cellar has slab on grade, Live Load of 100 psf.



1 New Beam to Existing

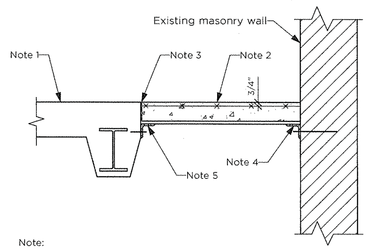


2 Beam Reinforcement
Scale: 1-1/2" = 1'-0"



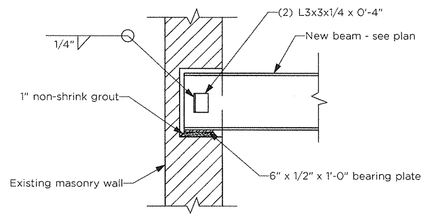
Note:
1. Existing floor slab not shown for clarity.
2. Remove existing encasement on existing trimmer for 6" on either side of the new connections.

3 New Beam to Existing



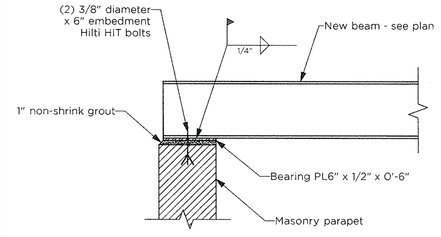
Note:
1. Existing concrete floor slab.
2. New 4" concrete slab with 6"x6" - W2.0 x W2.0 welded wire fabric on UF2X 2" 20-gage metal form deck.
3. Compressible joint.
4. New L3x3x3/8 shelf with 1/2"Ø Hilti HY20 bolts, 6" embedment, at 24" o.c.
5. New L3x3x3/8 shelf with 1/2"Ø x 6" embedment Hilti HIT HY150 bolts @ 36" o.c.

4 New Slab Infill

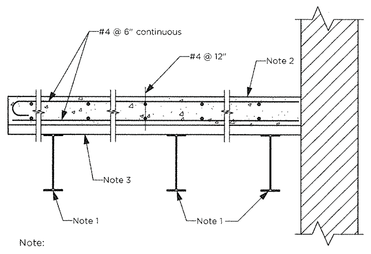


Note:
1. Fill pocket solid with masonry after beam has been installed.

5 Beam Bearing

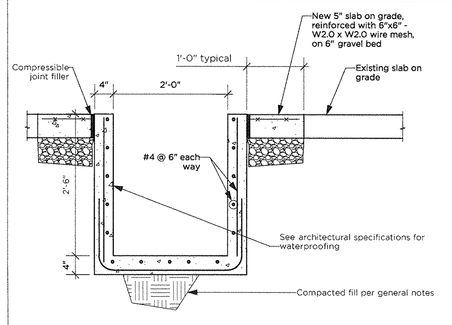


6 Dunnage Bearing

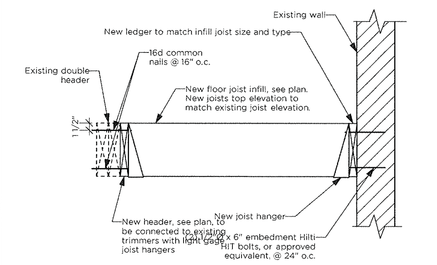


Note:
1. See plan for new beams and dimensions.
2. New concrete slab: 6" above deck.
3. New form deck - see plan. See Steel Deck Note 6 for shoring requirements.

7 New 2nd Floor Slab

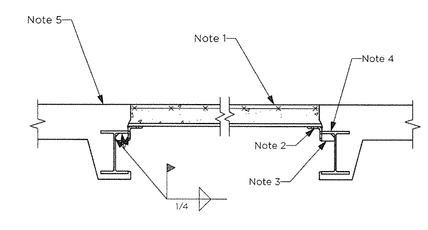


8 Sump Pit



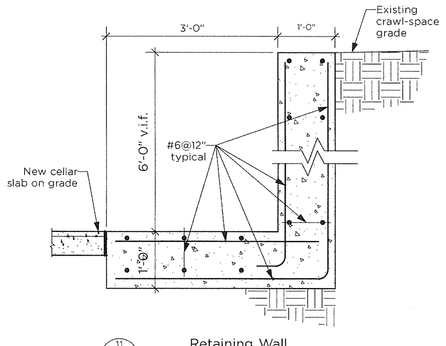
Note:
1. New subfloor (not shown for clarity) top elevation to match existing subfloor top elevation.

9 Wood Floor Infill

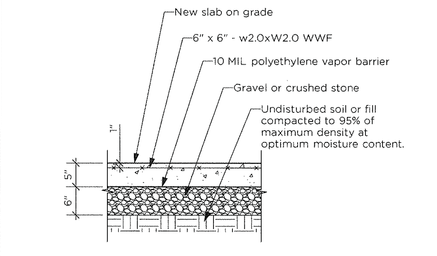


Note:
1. New 4" concrete slab with 6"x6" - W2.0 x W2.0 welded wire fabric on UF2X 2" 20-gage metal form deck.
2. L3x3x3/8 shelf angle.
3. 3/8" plate @ 16" o.c. max spacing.
4. Existing steel beam.
5. Existing concrete slab.

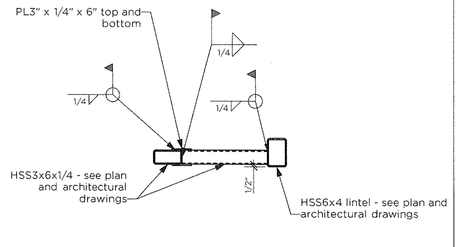
10 New Slab Infill



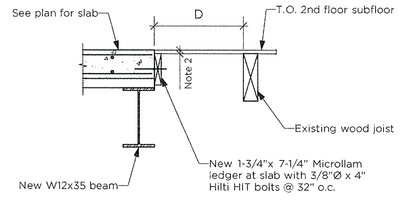
11 Retaining Wall



12 Slab on Grade



13 Canopy Frame



Note:
1. Detail is applicable if D > 6".
2. New ledger to be installed below the depth of existing subfloor and new slab.

14 Section at New Deck Floor

Project
BROOKLYN PHILHARMONIC MUSIC CENTER
299 DeGraw Street, Brooklyn, NY 11231

Prepared for
Brooklyn Philharmonic
55 Washington Street, Suite 656
Brooklyn, NY 11201

Contract No: 79.07250.09

HOK
1065 Avenue of the Americas
New York, NY 10018 USA
/ +1 212 741 1200 / +1 212 633 1163

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NEW YORK CITY DEPARTMENT OF DESIGN + CONSTRUCTION

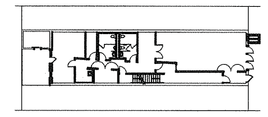
30-30 Thompson Avenue, #5-076
Long Island City, NY 11101

Structural Engineers
111 Broadway, 14th Floor

MEP Engineers
29 West 38th Street, 5th Floor

Lighting Consultant
11 West 42 Street

Key Plan



BLOCK: 414
LOT: 61

Professional Seals

It is a violation of law for any person, unless acting under the direction of a licensed architect, to alter any item on these drawings.

No.	Issue Description	MM-DD-YYYY
1	ISSUED FOR BID	07-20-2012

No.	Revision Description	MM-DD-YYYY

B-Scan

Drawn by SL Reviewed by DF
Project No 79.07250.09
Date June 1, 2011
CAPIS ID PV467BPHL

Sheet Title

PROJECT NOTES AND DETAILS

Sheet Number

S-200.00

Original drawing is 42 x 36 Do not scale contents of this drawing